# Regulating the Traffic Flow to $60^{\text {th }}$ Road Khartoum, Sudan using the Green Wave System 

Abdel Aziz Hassan Abdel Razig Ali¹, Adil. A.M. Elhassan ${ }^{2}$, Eltahir<br>Abuelgasem Mohamed ${ }^{3}$<br>${ }^{1}$ Assistant Professor, Sudan University of Science \& Technology (SUST), College of Engineering, Khartoum, Sudan<br>${ }^{2.1}$ Assistant Professor - Department of Civil Engineering, College of Engineering, Taif University (KSA)<br>${ }^{2.2}$ Assistant Professor -Sudan University of Science and Technology (SUST), Khartoum, Sudan<br>${ }^{3}$ Assistant Professor, Sudan University of Science \& Technology (SUST), College of Engineering, Khartoum, Sudan


#### Abstract

Sixtieth road is one of the main roads in Khartoum state - Capital of Sudan, and it's the widest road in the Khartoum state, the length of this road equal $(6,880.0)$ meters, and it has seven traffic lanes with two Medians, and it has ten surface intersections. Overlooking this road is the most famous high-end residential neighborhood in the state of Khartoum, and the volume of traffic on this road is relatively large. Sixty road extends from the intersection of Al-Manshiya Bridge in the north to the intersection of Madani road in the south, and due to the large number of surface intersections of this road, it was necessary to coordinate the traffic signals on it, in order to facilitate movement on this road without frequent stops.In this study, the information required to coordinate the light signals with the green wave system, which is the distance between successive intersections and the number of lanes at each intersection, was collected. Traffic volume was also measured at all intersections. The study found a design of the light signals for all intersections in the green wave system, with a length of 135 seconds.


KEYWORDS: Sixtieth road, green wave system, intersections, traffic signals

## I. INTRODUCTION

The most commonly-used linking system works with a cycle time common to all intersections and the signals are so timed that the 'go' periods are staggered in relation to each other according to the road speed to give a 'progression' of green periods along the road in both directions [1-6]. Thus road speed should be considered "reasonable" by drivers; if speeding is common before linking, and then a measured speed will be too high for safe operation. In this case, a desired speed should be used to ensure that the platoon conforms to the legal limit [7-10]. The timings of the signals in a simple progressive system can be prepared with the aid of a tunedistance diagram, examples of which are shown in Figures 1 and 2.


Figure 1 Co-ordinated signals for one-way traffic


Figure 2 Co-ordinated signals for two-way traffic
On these diagrams, distances between junctions along the route are plotted along the abscissa (y axis) and the travel times are plotted along the ordinate (x-axis). The slopes of diagonal lines represent the chosen speed of progression and green stages of successive junctions are offset in time. Normally the problem is one of determining, by trial and error, the optimum through-band speed and width for a fixed cycle time. For one-way roads, the green bands follow each other in sequence. The driver, having passed one intersection, will then receive right of way at the others.

When the flow of traffic is two-directional and where the intersections are not equally spaced, the situation is more complex and it may be necessary to come to a compromise on progression between the two directions. It may also be necessary to take into account other requirements such as demands from cross- road traffic. In heavy city center traffic a design `speed' of about ( $40 \mathrm{~km} / \mathrm{h}$ ) usually gives good results. For suburban traffic, where traffic is lighter and signals are about 300 m apart, a design velocity of about ( $60 \mathrm{~km} / \mathrm{h}$ ) can be used as a first estimate, provided this does not conflict with local speed restrictions. On two-way roads, good coordination can usually be obtained by using a common cycle time equivalent to twice the average travel time between junctions [11-13].

## II. LITERATURE SURVEY

$60^{\text {th }}$ road is one of the main roads in the Khartoum state - Sudan; this road is considered the widest road in the state, the length of this road is $6,880.0$ meters, the width of 7 traffic lanes, without the central medians, it has ten superficial intersections. It is located on this road, the most famous high-end residential neighborhood in the Khartoum state; traffic volume on this road is relatively large. $60^{\text {th }}$ road extends from the intersection of AlMansheya Bridge in the north direction to the intersection of Madani road in the south direction (Figure.3).


Figure 3 Screen shot of 60th road Gate from Google map
The field information was collected and results are shown in Table 1 and Figure 4.
*Table. 1 The number, names and shapes of intersections on the $60^{\text {th }}$ road.

| No | Intersection Name | Shape |
| :---: | :--- | :---: |
| 1 | Mansheya Bridge | T Section |
| 2 | Alsafarat | + Section |
| 3 | Omak | + Section |
| 4 | Almashtl | T Section |
| 5 | Mukah | T Section |
| 6 | Abdullah AlTayeb | T Section |
| 7 | Alshargi | + Section |
| 8 | Alblabil | T Section |
| 9 | juba | + Section |
| 10 | Madni road | T Section |




Figure 4 Screen shot of shapes of intersections on the $60^{\text {th }}$ road Gate from Google map

## III. METHODOLOGY

*Calculate the total length of the road and the distance between successive intersections (Table 2).
Table. 2 Length between the intersections

| From | To | Length (m) |
| :---: | :---: | :---: |
| Mansheya Bridge | Alsafarat | 490.0 |
| Alsafarat | Omak | 530.0 |
| Omak | Almashtl | 730.0 |
| Almashtl | Mukah | 400.0 |
| Mukah | Abdullah AlTayeb | 620.0 |
| Abdullah AlTayeb | Alshargi | $1,000.0$ |
| Alshargi | Alblabil | 750.0 |
| Alblabil | juba | $1,470.0$ |
| juba | Madni road | 890.0 |
| Total Length $(\mathbf{K m})$ |  | $\mathbf{6 , 8 8 0 . 0}$ |

*Calculating the number and width of lanes on $60^{\text {th }}$ road and the roads connected to it (Table 3).
Table. 3 number and width of lanes on $60^{\text {th }}$ road

| No | Intersection Name | width of lanes (m) |  |
| :---: | :--- | :---: | :---: |
|  |  | $\mathbf{6 0}^{\text {th }} \mathbf{S t .}$ | Roads |
| 1 | Mansheya Bridge | 17.5 | 14.0 |
| 2 | Alsafarat | 17.5 | 07.0 |
| 3 | Omak | 24.5 | 14.0 |
| 4 | Almashtl | 24.5 | 14.0 |
| 5 | Mukah | 24.5 | 07.0 |
| 6 | Abdullah AlTayeb | 24.5 | 07.0 |
| 7 | Alshargi | 24.5 | 07.0 |
| 8 | Alblabil | 21.0 | 14.0 |
| 9 | juba | 21.0 | 14.0 |
| 10 | Madni road | 21.0 | 14.0 |

[^0]Table.4.A Normal flow, Saturation flow on $60^{\text {th }}$ road

| Tree Phase (T Section, six Intersections)* |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| FROM | $\mathbf{N}$ |  | $\mathbf{W}$ | S |  |
| TO | $\mathbf{S}$ | $\mathbf{W}$ | $\mathbf{N \& S}$ | $\mathbf{N}$ | $\mathbf{W}$ |
| Q (Pcu/hr) | 2574 | 332 | 257 | 362 | 351 |
| S (Pcu/hr) | 5400 | 1800 | 1800 | 1800 | 1800 |

*The maximum traffic volume was chosen from among the six intersections
Table.4.B Normal flow, Saturation flow on $60^{\text {th }}$ road

| Four Phase (+ Section, four Intersections)* |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| FROM | N |  |  | S |  |  | E |  |  | W |  |  |
| TO | S | E | W | N | E | W | N | S | W | N | S | E |
| Q (Pcu/hr) | 224 | 450 | 347 | 420 | 153 | 210 | 152 | 256 | 233 | 284 | 241 | 243 |
| $\mathbf{S}$ (Pcu/hr) | 3600 | 1800 | 3600 | 1800 | 1800 | 1800 | 1800 | 1800 | 1800 | 1800 | 1800 | 1800 |

* The maximum traffic volume was chosen from among the four intersections


## IV. CALCULATION OF CYCLE LENGTH

## Three phase intersection

(Mansheya Bridge, Almashtl, Mukah, Abdullah AlTayeb, Alblabil and Madni road)

- $\mathrm{C} 0=(1.5 \mathrm{~L}+5) /(1-\mathrm{Y})$
- $\mathrm{L}=2 \mathrm{~N}+\mathrm{R}$
- $\quad \mathrm{G} 1=(\mathrm{Y} 1 / \mathrm{Y})(\mathrm{C} 0-\mathrm{L})$
- $\mathrm{G} 2=(\mathrm{Y} 1 / \mathrm{Y})(\mathrm{C} 0-\mathrm{L})$
- $\mathrm{G} 3=(\mathrm{Y} 1 / \mathrm{Y})(\mathrm{C} 0-\mathrm{L})$

Where:
$\checkmark \quad \mathrm{C} 0=$ Optimum cycle length (sec).
$\checkmark \quad \mathrm{L}=$ Total lost time per cycle (sec).
$\checkmark \quad \mathrm{Y}=$ Maximum value of the ratios (Q/S).
$\checkmark \quad \mathrm{N}=$ Number of phases.
$\checkmark \quad \mathrm{R}=$ All red time (sec).
$\checkmark \quad \mathrm{Q}=$ Normal flow (Pcu/hr).

### 4.2 Four phase intersection

(Alsafarat, Omak, Alshargi and juba)

|  | Pase (1) | Phase (2) | Phase (3) | Phase (4) |
| :--- | :---: | :---: | :---: | :---: |
|  | $N$ | $E$ | 5 | $W$ |
| $T G(s e c)$ | 38 | 22 | 35 | 24 |
| $T(\mathrm{G} / \mathrm{Y})(\mathrm{sec})$ | 2 | 2 | 2 | 2 |
| $\mathrm{~T}(\mathrm{R})(\mathrm{sec})$ | 90 | 106 | 93 | 104 |
| $\mathrm{~T}(\mathrm{Y})(\mathrm{sec})$ | 5 | 5 | 5 | 5 |


| INPUT DATA |  |  |
| :---: | :---: | :---: |
| $\mathrm{N}=$ | 3.0 | Sec |
| $\mathrm{R}=$ | 6.0 | Sec |
| $\mathrm{L}=$ | 12.0 | Sec |


| Phase.1 |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| From |  | N |  |  |
| To | S | W |  |  |
| $\mathrm{Q}(\mathrm{PCu} / \mathrm{hr})$ | 2574 | 332 |  |  |
| $\mathrm{~S}(\mathrm{Pcu} / \mathrm{hr})$ | 5400 | 1800 |  |  |
| $\mathrm{y}(\mathrm{q} / \mathrm{s})$ | 0.48 | 0.18 |  |  |
| Y max |  |  |  | 0.48 |


| Phase.2 |  |
| :---: | :---: |
| From | W |
| To | $\mathrm{N} \& \mathrm{~S}$ |
| $\mathrm{Q}(\mathrm{Pcu} / \mathrm{hr})$ | 257 |
| $\mathrm{~S}(\mathrm{Pcu} / \mathrm{hr})$ | 1800 |
| $\mathrm{y}(\mathrm{q} / \mathrm{s})$ | 0.14 |
| Y max | 0.14 |


| Phase. 3 |  |  |
| :---: | :---: | :---: |
| From |  | S |
| To | N | W |
| Q (P¢U/hr) | 362 | 351 |
| $\mathrm{S}(\mathrm{Pcu} / \mathrm{hr})$ | 1800 | 1800 |
| $y(q / s)$ | 0.20 | 0.20 |
| Y max |  | 0.20 |
| $\Sigma \mathrm{Y}$ max $=$ | 0.82 |  |
| $\mathrm{CO}=$ | 128 | Sec |
| Take $\mathrm{CO}=$ | 135 | Sec |
| G. $1=$ | 71 | Sec |
| G.2= | 21 | Sec |
| G.3= | 30 | Sec |


|  | Pase (1) | Phase (2) | Phase (3) |
| :---: | :---: | :---: | :---: |
|  | $N$ | $W$ | $S$ |
| $T G(\mathrm{sec})$ | 71 | 21 | 30 |
| $\mathrm{~T}(\mathrm{G} / \mathrm{Y})(\mathrm{sec})$ | 2 | 2 | 2 |
| $\mathrm{~T}(\mathrm{R})(\mathrm{sec})$ | 57 | 107 | 98 |
| $\mathrm{~T}(\mathrm{Y})(\mathrm{sec})$ | 5 | 5 | 5 |

## V. DISCUSSION OF RESULTS

The (T-section) intersections (6 intersections) were designed with a three-phase traffic light due to the presence of three arms, total cycle time $=130$ seconds, while the ( + section) intersections ( 4 intersections) were designed with a four-phase traffic light because they consist of four arms, the total cycle time $=135$ seconds, it was chosen that the total cycle time for all intersections equal to 135 seconds (Table 5).

Table. 5 Cycle length for green wave

| No | Intersection Name | Shape | Cycle length (from <br> design) <br> (Sec). | Cycle length (for <br> green wave) <br> (Sec). |
| :---: | :--- | :---: | :---: | :---: |
| 1 | Mansheya Bridge | T Section | 130 | 135 |
| 2 | Alsafarat | + Section | 135 | 135 |
| 3 | Omak | + Section | 135 | 135 |
| 4 | Almashtl | T Section | 130 | 135 |
| 5 | Mukah | T Section | 130 | 135 |
| 6 | Abdullah AlTayeb | T Section | 130 | 135 |
| 7 | Alshargi | + Section | 135 | 135 |
| 8 | Alblabil | T Section | 130 | 135 |
| 9 | juba | + Section | 135 | 135 |
| 10 | Madni road | T Section | 130 | 135 |

According to the distance between the ten intersections (in meters) and considering that the average speed of walking on this road is $(50 \mathrm{Km} / \mathrm{hr})$, the time of the green wave between the intersections was determined, which is as shown in Table 6.

Table. 6 Time between intersections (sec)

| From | To | Distance <br> $(\mathbf{m})$ | Speed <br> $(\mathbf{K m} / \mathbf{h r})$ | Time between <br> intersections (sec) |
| :---: | :---: | :---: | :---: | :---: |
| Mansheya Bridge | Alsafarat | 490.0 | 50.0 | 35 |
| Alsafarat | Omak | 530.0 | 50.0 | 38 |
| Omak | Almashtl | 730.0 | 50.0 | 53 |
| Almashtl | Mukah | 400.0 | 50.0 | 29 |
| Mukah | Abdullah AlTayeb | 620.0 | 50.0 | 45 |
| Abdullah AlTayeb | Alshargi | 1000.0 | 50.0 | 72 |
| Alshargi | Alblabil | 750.0 | 50.0 | 54 |
| Alblabil | juba | 1470.0 | 50.0 | 106 |
| juba | Madni road | 890.0 | 50.0 | 64 |

## VI. CONCLUSION

From the results of this study, the following conclusions have been drawn:

- The total length of $60^{\text {th }}$ road is equal to $6,880.0$ meters, and the number of its intersections is equal to ten surface intersections.
- The distance between intersections is unequal.
- The light signals are all working, but need to modify the geometric design to match the design of the green wave signal.
- There are four traffic signals that need maintenance and rehabilitation.
- Standardization of the traffic signal cycle time for similar intersections.
- Improving the geometric design at all intersections in $60^{\text {th }}$ road in order for the green wave system to function properly.


## REFERENCES

[1] Department of Transport. Junction Layout for Control by Traffic Signals. Highway, Safety and Traffic Advice Note TA 18/81. Department of Transport. 1992.
[2] Fouracre, P. R. and Gardner,G. Traffic Signals in Developing Cities, Overseas Unit Working Paper. Transport and Road Research Laboratory. 1990.
[3] Willumsen, L.G. and Coeymans, J.E. Research into the Value of Area Traffic Control Techniques in a Developing Contry. TRRL Contractor Report 99. Transport and Road Research Laboratory. 1988.
[4] TRL. Urban Road Trafic Surveys. Overseas Road Note 11. Overseas Centre, Transport Research Laboratory. 1993.
[5] Jourdain, S. Urban Intersection Control. The Book Guild Ltd. Lewes, Sussex. 1992.
[6] Institution of Highways and Transportation/ Department of Transport. Roads and Traffic in Urban Areas. HMSO, London. 1987.
[7] Highway Engineering- S. K. Khanna \& C. E. G Justo, Nemchand \& Bros., 7th Edition.
[8] Traffic Engineering \& Transportation Planning- L.K Kadiyali- Khanna Publications.
[9] Dowling, R. Gv D. B. Reinke, A. Flannery, P. Ryus, M.Vandehey, T. A. Petritsch, B. W. Landis, N.M.Rouphail, and J. A. Bonneson. NCHRP Report 616: Multimodal Level of Service Analysis for Urban Streets. Transportation ResearchBoard of the National Academies, Washington, D.C., 2008.
[10] Adil A. M. Elhassan1 and Ahmed Abdelhafiz. Development of Downtown Traffic System of Taif City, Journal of Transportation Systems, Volume 4 Issue 3 Page 1-8 © MAT Journals 2019.
[11] Fang, F., and K. K. Pecheux. Analysis of User Perception of Level of Service UsingFuzzy Data MiningTechnique. Presented at 86th Annual Meeting of the Transportation Research Board, Washington, D.C., 2007.
[12] Manual on Uniform Traffic Control Devicesfor Streets and Highways. Federal Highway Administration, Washington, D.C., 2009. http://mutcd.fhwa.dot.gov. Accessed Feb. 1, 2010.
[13] HetalB.Patel and Bhasker Vijaykumar Bhatt. A critical study of road intersections in the south east part of Surat city, Volume 5, Issue 4 www.jetir.org (ISSN-2349-5162)., 2018.


[^0]:    * Traffic survey's/count shown in Tables 4.A and 4.B

